

DPG method with optimal test functions for thin-body problems in solid mechanics

Ludwig Mitter



ludwig.mitter@numa.uni-linz.ac.at

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DPG thin-body

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Dimensional Reduction in Continuum Mechanics

DPG thin-body

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Why dimensional reduction for thin bodies?

1. Avoid **geometry locking** with standard discretization
2. Avoid very **complex three-dimensional discretization** that is stable wrt. thickness
3. Replace three-dimensional problem with **two- or one-dimensional problem**

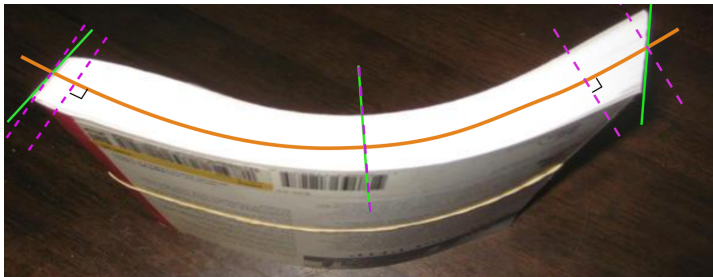
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Dimensional Reduction in Continuum Mechanics

How does it work?

1. Claim that three-dimensional displacement has specific form (**lower-dimensional parameter functions**)
2. Insert ansatz into three-dimensional model and **perform calculations analytically** as much as possible

What do we get?

BEAMS Straight 1D bodies

ARCHES Curved 1D bodies

PLATES Planar 2D bodies

SHELLS Curved 2D bodies

The Timoshenko beam model

Displacement

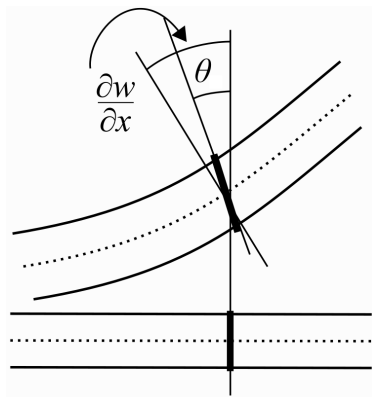
$$\begin{bmatrix} u_1, u_2, u_3 \end{bmatrix} (x_1, x_2, x_3) = \begin{bmatrix} -x_3 \theta(x_1), 0, w(x_1) \end{bmatrix}^T$$

w ...transverse deflection

θ ...rotation

Perpendicular loading

$$\begin{bmatrix} f_1, f_2, f_3 \end{bmatrix} (x_1, x_2, x_3) = \begin{bmatrix} 0, 0, p(x_1) \end{bmatrix}^T$$



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Derivation of the Timoschenko system

$$\mathbf{E} = \begin{bmatrix} -x_3 \theta'(x_1) & 0 & \frac{1}{2}(w'(x_1) - \theta(x_1)) \\ 0 & 0 & 0 \\ \frac{1}{2}(w'(x_1) - \theta(x_1)) & 0 & 0 \end{bmatrix}$$

Modified constitutive law for **consistency**.

$$\mathbf{\Sigma} = \begin{bmatrix} -Ex_3 \theta'(x_1) & 0 & \kappa \mu (w'(x_1) - \theta(x_1)) \\ 0 & -E\nu x_3 \theta'(x_1) & 0 \\ \kappa \mu (w'(x_1) - \theta(x_1)) & 0 & 0 \end{bmatrix}$$

Assume $\Omega = (0, L) \times \omega$. Set $I := \int_{\omega} x_3^2 dx_3$, $A := |\omega|$.

$$\begin{aligned} (\mathbf{\Sigma}, \mathbf{E})_{0, \Omega} &= EI \int_a^b \theta'(x_1)^2 dx_1 \\ &\quad + \kappa \mu A \int_a^b (w'(x_1) - \theta(x_1))^2 dx_1 \end{aligned}$$

The Timoschenko system

Find w, θ such that

$$\begin{aligned} -\frac{\partial}{\partial x_1} \left[EI \frac{\partial \theta}{\partial x_1} \right] + \kappa \mu A \left(\frac{\partial w}{\partial x_1} - \theta \right) &= m \text{ in } (0, L) \\ -\frac{\partial}{\partial x_1} \left[\kappa \mu A \left(\frac{\partial w}{\partial x_1} - \theta \right) \right] &= p \text{ in } (0, L) \end{aligned}$$

Introducing auxiliary variables and rescaling

1. Auxiliary variables:

$$V = \kappa \mu A (w' - \theta) \quad M = EI \theta'$$

leads to

$$-V' = p \quad -M' - V = m$$

2. Rescaling:

$$x_1 \mapsto Lx_1 \quad w \mapsto Lw \quad \theta \mapsto \theta$$

$$V \mapsto \mu AV \quad M \mapsto EIL^{-1}M \quad p \mapsto \mu AL^{-1}p \quad m \mapsto EIL^{-2}m$$

leads to **dimensionless system**

$$V = \kappa (w' - \theta) \quad M = \theta' \text{ in } (0, L)$$

$$-V' = p \quad -M' - kV = m \text{ in } (0, L)$$

$$\text{where } k = \frac{\mu AL^2}{EI}$$

Assumptions

1. Crosssection of beam is **rectangle** (width b , thickness d)

$$\implies A = bd, I = \frac{bd^3}{12}, k = \frac{12\mu\epsilon^2}{E}$$

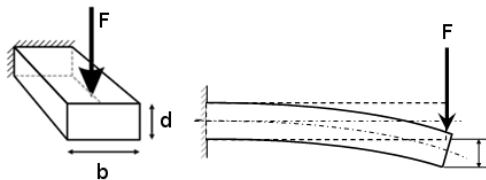
where $\epsilon = \frac{d}{L}$ **dimensionless thickness**

2. Assume **thin beam** $\epsilon \ll 1$

\implies **rescaling** $V \mapsto \epsilon^{-2}V, p \mapsto \epsilon^{-2}p$ which leads to

$$\begin{aligned} \epsilon^{-2}V &= \kappa(w' - \theta) & M &= \theta' \text{ in } (0, 1) \\ -V' &= p & -M' - kV &= m \text{ in } (0, 1) \end{aligned}$$

where $k = \frac{12\mu}{E}$



The Model Problem

1. **Volume loads:** $p \equiv 0, m \equiv 0$
2. **Surface load** at $\{L\} \times \omega$ of magnitude $(0, 0, F)^\top$
3. **Clamped** at $\{0\} \times \omega$

This leads to the

MODEL PROBLEM

Find V, M, w, θ such that

$$\begin{aligned} \epsilon^{-2}V &= \kappa(w' - \theta) & M &= \theta' \text{ in } (0, 1) \\ -V' &= p & -M' - kV &= m \text{ in } (0, 1) \\ V(1) &= F & M(1) &= 0 & w(0) &= 0 & \theta(0) &= 0 \end{aligned}$$

where $k = \frac{12\mu}{E}$

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This problem can be solved explicitly:

$$V = F \quad M = -kF(x_1 - 1)$$

$$\theta = kF \left(x_1 - \frac{1}{2}x_1^2 \right) \quad w = \frac{kF}{2} \left(x_1^2 - \frac{x_1^3}{3} \right) + \frac{F}{\epsilon^2 \kappa} x_1$$

The weak ONE-ELEMENT formulation

Find $\mathbf{u} = (V, M, w, \theta) \times (\hat{V}(0), \hat{M}(0), \hat{w}(1), \hat{\theta}(1)) \in \mathcal{U}$ such that

$$\epsilon^2(V, q) + \kappa(w, q') - \kappa \hat{w}(1)q(1) + \kappa(\theta, q) = 0$$

$$(M, \tau) + (\theta, \tau') - \hat{\theta}(1)\tau(1) = 0$$

$$(V, z') + \hat{V}(0)z(0) = Fz(1)$$

$$k(V, \phi) - (M, \phi') - \hat{M}(0)\phi(0) = 0$$

for all $\mathbf{v} = (q, \tau, z, \phi) \in \mathcal{V}$, where $(a, b) := \int_0^1 ab dx_1$ and $\mathcal{U} := [L^2(0, 1)]^4 \times \mathbb{R}^4$, $\mathcal{V} = [H^1(0, 1)]^4$.

Short form:

$$\text{Find } \mathbf{u} \in \mathcal{U} : B(\mathbf{u}, \mathbf{v}) = L(\mathbf{v}), \forall \mathbf{v} \in \mathcal{V}$$

1. Equip Hilbert space \mathcal{U} with *energy norm*

$$\|\mathbf{u}\| := \sup_{\mathbf{v} \in \mathcal{V}} \frac{B(\mathbf{u}, \mathbf{v})}{\|\mathbf{v}\|_{\mathcal{V}}}$$

2. Regular Hilbert space norm $\|\cdot\|_{\mathcal{V}}$ corresponding to $(\cdot, \cdot)_{\mathcal{V}}$

$$\|\mathbf{v}\|_{\mathcal{V}}^2 = \|q\|_V^2 + \|\tau\|_V^2 + \|z\|_V^2 + \|\phi\|_V^2 \quad \|\mathbf{v}\|_V^2 = \|\mathbf{v}\|^2 + \|\mathbf{v}'\|^2$$

3. **Reminder:** Then one has $\|\mathbf{u}\| = \|T\mathbf{u}\|_{\mathcal{V}}$ with $T\mathbf{u}$ as

$$(T\mathbf{u}, \mathbf{v})_{\mathcal{V}} = B(\mathbf{u}, \mathbf{v}), \forall \mathbf{v} \in \mathcal{V}$$

4. Equip Hilbert space \mathcal{U} with “more” standard norm

$$\|\mathbf{u}\|_{\mathcal{U}} := \max\{\|V\|, \|M\|, \|w\|, \|\theta\|, |\hat{V}(0)|, |\hat{M}(0)|, |\hat{w}(1)|, |\theta(\hat{1})|\}$$

Theorem

Let $\mathbf{u} \in \mathcal{U}$. Then there exist two constants $c_1, c_2 > 0$ independent of ϵ such that

$$c_1 \|\mathbf{u}\|_{\mathcal{U}} \leq \|\mathbf{u}\| \leq c_2 \|\mathbf{u}\|_{\mathcal{U}}.$$

Hence, $B(.,.)$ is bounded and satisfies inf-sup.

Proof.

Construction of an explicit expression for the energy norm
AND replacement of $\|\cdot\|_V = \|\cdot\|_{H^1}$ above with

$$\|v\|_V^2 := \|v'\|^2 + |v(1)|^2$$

Blackboard.



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1. Let $\mathcal{U}_n = \text{span}\{\mathbf{e}_1, \dots, \mathbf{e}_n\} \subset \mathcal{U}$
2. Optimal test space $\mathcal{V}_n^{\text{opt}} = \text{span}\{T\mathbf{e}_1, \dots, T\mathbf{e}_n\}$
3. **Symmetrized PG method** (energy projection)

$$\text{Find } \mathbf{u}_n \in \mathcal{U}_n : B(\mathbf{u}_n, \mathbf{v}_n) = L(\mathbf{v}_n), \forall \mathbf{v}_n \in \mathcal{V}_n^{\text{opt}}.$$

Reminder:

$$\mathbf{v}_n = T\mathbf{w}_n, \mathbf{w}_n \in \mathcal{U}_n \implies (T\mathbf{u}_n, T\mathbf{w}_n) = L(T\mathbf{w}_n)$$

4. **Cea:**

$$\|\mathbf{u} - \mathbf{u}_n\|_{\mathcal{U}} \leq \frac{c_1}{c_2} \min_{\mathbf{w}_n \in \mathcal{U}_n} \|\mathbf{u} - \mathbf{w}_n\|_{\mathcal{U}}$$

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DPG with optimal test functions

Now the **ULTRA-weak formulation**

1. Additionally to

$$\mathbf{u}_n = (V, M, w, \theta) \times (\hat{V}(0), \hat{M}(0), \hat{w}(1), \hat{\theta}(1)) \in \mathcal{U}_n$$

include **interface variables**

$$\boldsymbol{\lambda}_n = (\hat{\mathbf{V}}, \hat{\mathbf{M}}, \hat{\mathbf{w}}, \hat{\boldsymbol{\theta}}) \in \Lambda_n = \mathbb{R}^{4N-4} \text{ corresponding to}$$

$$\mathcal{T}_n : 0 = x_0 < x_1 < \dots < x_N = 1$$

2. **DPG bilinear form** where

$$\mathbf{v}|_K = (q_j, \tau_j, z_j \phi_j) \in \mathcal{V}(K) = [H^1(K)]^4:$$

$$\begin{aligned} B_h(\mathbf{u}_n, \boldsymbol{\lambda}_n; \mathbf{v}) = & \sum_{j=1}^N \left[\epsilon^2 \int_{x_{j-1}}^{x_j} V q_j dx + \kappa \int_{x_{j-1}}^{x_j} w q_j' dx \right. \\ & - \kappa \hat{w}(x) q_j(x) \Big|_{x_{j-1}}^{x_j} + \kappa \int_{x_{j-1}}^{x_j} \theta q_j dx + \int_{x_{j-1}}^{x_j} M \tau_j dx + \int_{x_{j-1}}^{x_j} \theta \tau_j' dx \\ & - \hat{\theta}(x) \tau_j(x) \Big|_{x_{j-1}}^{x_j} + \int_{x_{j-1}}^{x_j} V z_j' dx - \hat{V}(x) z_j(x) \Big|_{x_{j-1}}^{x_j} + k \int_{x_{j-1}}^{x_j} V \phi_j dx \\ & \left. - \int_{x_{j-1}}^{x_j} M \phi_j' dx + \hat{M}(x) \phi_j(x) \Big|_{x_{j-1}}^{x_j} \right] \end{aligned}$$

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1. **Discont. test space:** $\|\mathbf{v}\|_{\mathcal{V}_n} := \sum_K \|\mathbf{v}\|_{\mathcal{V}(K)}$

$$\mathcal{V}_n := \{\mathbf{v} \in [L^2(0,1)]^4, \mathbf{v}|_K \in \mathcal{V}(K), K \in \mathcal{T}_n\}$$

2. **New optimal test space:**

$$\mathcal{W}_n^{\text{opt}} = \text{span}\{T_n \mathbf{e}_1, \dots, T_n \mathbf{e}_n, T_n \boldsymbol{\lambda}_n\} \text{ where}$$

$$T_n : \mathcal{U}_n \times \Lambda_n \rightarrow \mathcal{V}_n \text{ such that}$$

$$(T_n(\mathbf{u}_n, \boldsymbol{\lambda}_n), \mathbf{v})_{\mathcal{V}_n} = B_n(\mathbf{u}_n, \boldsymbol{\lambda}_n; \mathbf{v}), \forall \mathbf{v} \in \mathcal{V}_n$$

DPG problem

Find $\mathbf{u}_n \in \mathcal{U}_n, \boldsymbol{\lambda}_n \in \Lambda_n$ such that

$$B_n(\mathbf{u}_n, \boldsymbol{\lambda}_n; \mathbf{v}) = L_n(\mathbf{v}), \forall \mathbf{v} \in \mathcal{W}_n^{\text{opt}}$$

The LOCALIZATION PRINCIPLE

Reminder

1. Element-wise computation of $\mathcal{W}_n^{\text{opt}}$
2. $B_n(\mathbf{u}_n, \boldsymbol{\lambda}_n; \mathbf{v}) = B(\mathbf{u}_n, \mathbf{v})$ and $\|\mathbf{v}\|_{\mathcal{V}_n} = \|\mathbf{v}\|_{\mathcal{V}}$ for $\mathbf{v} \in \mathcal{V}$

Lemma (Localization principle)

One has

$$\mathcal{V}_n^{\text{opt}} \subset \mathcal{W}_n^{\text{opt}},$$

consequently **ONE-ELEMENT** and **DPG** coincide.

Theorem (Best-approx. property)

1. $\mathbf{u} = (V, M, w, \theta)$ **one-element weak solution**
2. $\mathbf{u}_n = (V_n, M_n, w_n, \theta_n)$ **DPG solution**

Then \mathbf{u}_n is the **best-approx.** of \mathbf{u} wrt. $\|\cdot\|_{L^2}$ up to ϵ -indep. constant.

Remarks:

1. Optimal test fncts. corresponding to polynomial trial fncts. are NOT polynomials. Hence, resolve opt. test fncts. in **enriched FE space** (pol. degree +1)
2. Theoretical analysis of the **use of approx. opt. test fncts. is open** at the moment.
3. Numerics only for *Euler-Bernoulli* beam at $\epsilon = 0$ and $\epsilon = 0.1$, $\kappa = 5/6$ with **pw. const** and **pw. linear** elements.

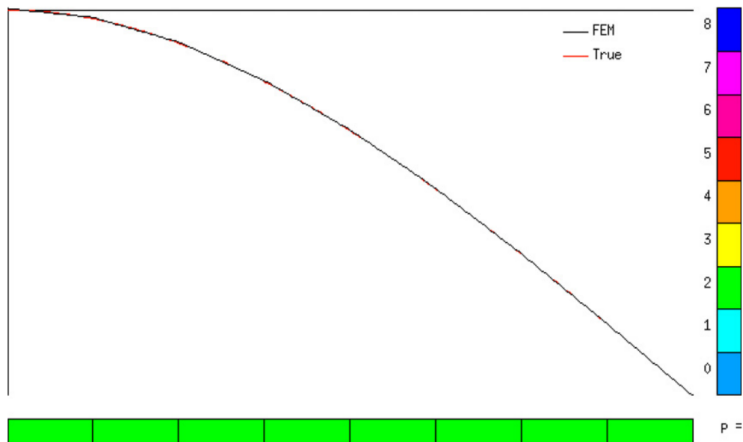


Figure: Transverse deflection w at $\epsilon = 0$.

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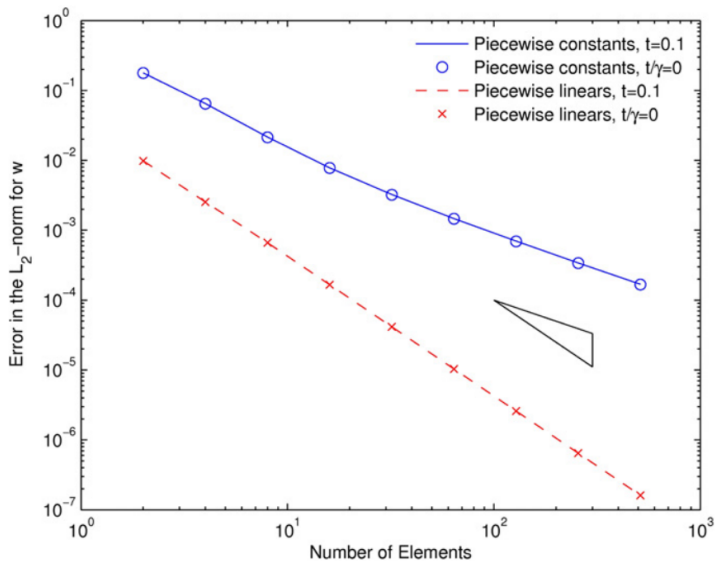


Figure: Convergence history for pw. constant and linear elements.

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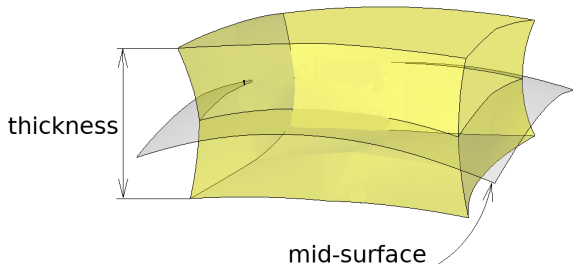
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Basic edge effect in shell deformation: ASSUMPTIONS

NOW: Shells (RN shell model). Assume

1. *Axially symmetric* deformations of a *shallow spherical shell* with radius R and thickness d , $d \ll R$.
2. If x_1, x_2 mid-surface coords, NO deformation along x_2 .
3. Homogeneous isotropic material.

THEN kinematics of shell given via single tangential displacement $u(x_1)$, transverse deflection $w(x_1)$ and single rotation of normal $\theta(x_1)$.



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If one takes R as *length unit*, EdR as *force unit*, dimless.
thickness $\epsilon = d/R$ one ends up with

Static equilibrium for Stresses T , Shear reaction V_x ,
Bending reaction M_x :

$$T_x = u' + w \quad T_y = w$$

$$V_x = \kappa(w' - \theta) \quad M_x = \frac{\epsilon^2}{12}\theta'$$

$$-T'_x = 0 \quad T_x + T_y - V'_x = 0 \quad -M'_x - V_x = 0$$

1. **Assume:** $T_x = 0$
2. **Rescaling:** $V_x \leftrightarrow \epsilon^2 V, M_x \leftrightarrow \epsilon^2 k^{-1} M, k = 12$

MODEL PROBLEM

Find V, M, w, θ such that

$$\begin{aligned}\epsilon^2 V &= \kappa(w' - \theta) & M &= \theta' \\ w - \epsilon^2 V' &= 0 & -M' - kV &= 0 \\ M(0) &= 1 & V(0), w(1), \theta(1) &= \dots\end{aligned}$$

where $k = 12$

Cannot be resolved by direct elimination of the unknowns,
however one can show

$$\mathbf{u}(x_1) = e^{\lambda x_1} \mathbf{U}$$

$$\implies \mathbf{u}(x_1) = e^{ax_1} \left(\mathbf{A}(\epsilon) \cos(bx_1) + \mathbf{B}(\epsilon) \sin(bx_1) \right)$$

$$\text{where } a \approx b \approx \pm 3^{1/4} / \sqrt{\epsilon}$$

The weak ONE-ELEMENT formulation

Similar setting as for Timoschenko-beam model:

$$\mathbf{u} = (V, M, \theta, w) \times (\hat{w}(0), \hat{\theta}(0), \epsilon^2 \hat{V}(1), \hat{M}(1)) \in \mathcal{U}$$

$$\mathbf{v} = (q, \tau, z, \phi) \in \mathcal{V}$$

$$\mathcal{U} = [L^2(0, 1)]^4 \times \mathbb{R}^4 \quad \mathcal{V} = [H^1(0, 1)]^4$$

PG formulation

$$\text{Find } \mathbf{u} \in \mathcal{U} : B(\mathbf{u}, \mathbf{v}) = L(\mathbf{v}), \forall \mathbf{v} \in \mathcal{V},$$

where

$$\begin{aligned} B(\mathbf{u}, \mathbf{v}) = & \epsilon^2 (V, q) + \kappa (w, q') + \kappa \hat{w}(0) q(0) + \kappa (\theta, q) \\ & + (M, \tau) + (\theta, \tau') + \hat{\theta}(0) \tau(0) + (w, z) \\ & + \epsilon^2 (V, z') - \epsilon^2 \hat{V}(1) z(1) \\ & + k (V, \phi) - (M, \phi') + \hat{M}(1) \phi(1) \end{aligned}$$

Choosing proper norms for ϵ -robustness

1. Timoschenko- $\|\cdot\|_{\mathcal{V}}$ DOES NOT lead to equivalence of $\|\cdot\|$ to $\|\cdot\|_{L^2}$ uniformly in ϵ .
2. **INSTEAD:** Arrive at $\|\cdot\| = \|\cdot\|_{\mathcal{U}}$ by setting

$$\|\mathbf{v}\|_{\mathcal{V}} := \sup_{\mathbf{u} \in \mathcal{V}} \frac{B(\mathbf{u}, \mathbf{v})}{\|\mathbf{u}\|_{\mathcal{U}}}$$

3. For our computations observe that

$$\begin{aligned} B(\mathbf{u}, \mathbf{v}) = & \left(V, \epsilon^2 q + \epsilon^2 z' + k\phi \right) + \left(M, \tau - \phi' \right) \\ & + \left(w, \kappa q' + z \right) + \left(\theta, \kappa q + \tau' \right) + \kappa \hat{w}(0)q(0) \\ & \hat{\theta}(0)\tau(0) - \epsilon^2 \hat{V}(1)z(1) + \hat{M}(1)\phi(1) \end{aligned}$$

hence by using $\|\cdot\|_{\mathcal{U}} = \max\{\dots\}$ one has (1-elem. case)

$$\begin{aligned} \|\mathbf{v}\|_{\mathcal{V}}^2 = & \|\epsilon^2(q + z') + k\phi\|^2 + \|\tau - \phi'\|^2 + \|\kappa q' + z\|^2 \\ & + \|\kappa q + \tau'\|^2 + |\kappa q(0)|^2 + |\tau(0)|^2 + |z(1)|^2 + |\phi(1)|^2 \end{aligned}$$

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1. Replace with **localizable norm**

$$\begin{aligned}\|\mathbf{v}\|_{\mathcal{V}}^2 = & \|\epsilon^2(q + z') + k\phi\|^2 + \|\tau - \phi'\|^2 + \|\kappa q' + z\|^2 \\ & + \|\kappa q + \tau'\|^2 + \|\kappa q\|^2 + \|\tau\|^2 + \|z\|^2 + \|\phi\|^2\end{aligned}$$

2. **Question remains open:** Are these two norms uniformly equivalent wrt. ϵ for DPG setting? (Numerics indicate robustness)
3. **OBSERVE:** Using above **optimal test space norm** instead of **standard test space norm** (as for Timoschenko), **boundary layer effects** have to be resolved by **test fncts.!** Hence, higher enrichment degree for FEM approx. of test fncts.

Remarks:

1. Trial fncts. are pw. pols. of equal order.
2. Optimal test space degree enrichment (+3).
3. Use both: **Optimal/Standard test space norms.**
4. Adaptive refinement (Endtmayer)

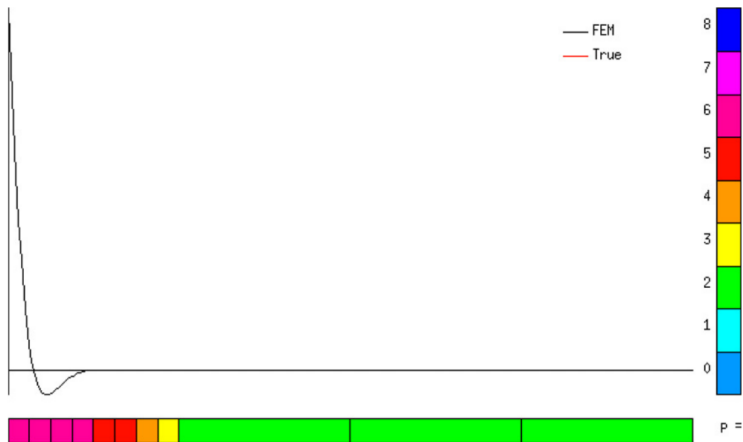


Figure: Bending moment M at $\epsilon = 1.e - 3$. OPTIMAL test space norm.

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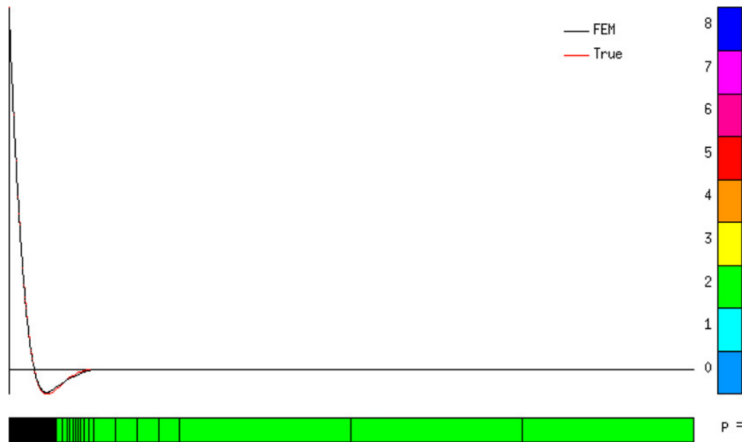


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Why could DPG methods be interesting for shell simulations?

1. Various **locking** effects disturb **FEM modelling of shells**.
2. **Beams, Arches, Plates** have a **relatively simple asymptotic behaviour** for $\epsilon \rightarrow 0$
3. **SHELL-asymptotics difficult**: Breaks into several subproblems (each with its own asymptotics and characteristic locking phenomena)
4. **Example**: Classical MITC4-S element attempts to cover these subproblems simultaneously.
5. **Can DPG better adapt to these subproblems simultaneously?**

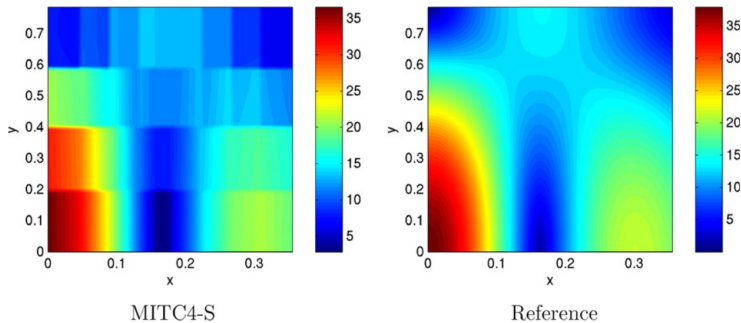


Figure: Comparison of MITC4-S to reference solution.

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formulation
DPG formulation
Numerics

Conclusions and
discussion